

Compression perpendicular to grain (bearing)

$$F_{c\perp u} = F_{c\perp} (K_F) = 565 \text{ psi} (2.083) = 1177 \text{ psi} = 1.177 \text{ ksi}$$

$$F_{c\perp u}^* = F_{c\perp u} (\Phi_c)(\lambda) = 1.177 \text{ ksi} (0.9)(0.8) = 0.847 \text{ ksi}$$

Compression parallel to grain

$$F_{cu} = F_c (K_F) = 1800 \text{ psi} (2.4) = 4320 \text{ psi} = 4.32 \text{ ksi}$$

$$F_{cu}^* = F_{cu} (\Phi_c)(\lambda) = 4.32 \text{ ksi} (0.9)(0.8) = 3.11 \text{ ksi}$$

$$P_u^* = F_{cu}^* (A) = 3.11 \text{ ksi} (39.38 \text{ in}^2) = 122 \text{ k}$$

Note that C_F would also apply for column buckling of an axially loaded member.

Modulus of Elasticity

$$E^* = E = 1,800,000 \text{ psi} = 1800 \text{ ksi}$$

$$E_{min-u} = E_{min} (K_F) = 660,000 \text{ psi} (1.765) = 1,165,000 \text{ psi} = 1165 \text{ ksi}$$

$$E_{min-u}^* = E_{min-u} (\Phi_s) = 1165 \text{ ksi} (0.85) = 990 \text{ ksi}$$

- c. **2 x 10 purlins at 4 ft o.c.; 1.2D + 1.6L_r load combination ($\lambda = 0.8$)**

C_F is already incorporated in NDS Table 4B values; other factors equal to unity

$$A = 13.88 \text{ in}^2; S_x = 21.39 \text{ in}^3$$

Bending moment

$$F_{bu} = F_b (K_F) = 2050 \text{ psi} (2.54) = 5207 \text{ psi} = 5.21 \text{ ksi}$$

$$F_{bu}^* = F_{bu} (\Phi_b)(\lambda) = 5.21 \text{ ksi} (0.85)(0.8) = 3.54 \text{ ksi}$$

$$M_u^* = F_{bu}^* (S_x) = 3.54 \text{ ksi} (21.39 \text{ in}^3) = 75.8 \text{ k-in.}$$

Tension

$$F_{tu} = F_t (K_F) = 1100 \text{ psi} (2.7) = 2970 \text{ psi} = 2.97 \text{ ksi}$$

$$F_{tu}^* = F_{tu} (\Phi_t)(\lambda) = 2.97 \text{ ksi} (0.8)(0.8) = 1.90 \text{ ksi}$$

$$T_u^* = F_{tu}^* (A) = 1.90 \text{ ksi} (13.88 \text{ in}^2) = 26.4 \text{ k}$$

Flexural Shear

$$F_{vu} = F_v (K_F) = 175 \text{ psi} (2.88) = 504 \text{ psi} = 0.504 \text{ ksi}$$

$$F_{vu}^* = F_{vu} (\Phi_v)(\lambda) = 0.504 \text{ ksi} (0.75)(0.8) = 0.302 \text{ ksi}$$

$$V_u^* = F_{vu}^* (2/3)(A) = 0.302 \text{ ksi} (2/3)(13.88 \text{ in}^2) = 2.8 \text{ k}$$

Compression perpendicular to grain (bearing)

$$F_{c\perp u} = F_{c\perp} (K_F) = 565 \text{ psi} (2.083) = 1177 \text{ psi} = 1.177 \text{ ksi}$$

$$F_{c\perp u}^* = F_{c\perp u} (\Phi_c)(\lambda) = 1.177 \text{ ksi} (0.9)(0.8) = 0.847 \text{ ksi}$$

Compression parallel to grain

$$F_{cu} = F_c (K_F) = 1850 \text{ psi} (2.4) = 4440 \text{ psi} = 4.44 \text{ ksi}$$

$$F_{cu}^* = F_{cu} (\Phi_c)(\lambda) = 4.44 \text{ ksi} (0.9)(0.8) = 3.20 \text{ ksi}$$

$$P_u^* = F_{cu}^* (A) = 3.20 \text{ ksi} (13.88 \text{ in}^2) = 44.4 \text{ k}$$

Note that C_F would also apply for column buckling of an axially loaded member.

Modulus of Elasticity

$$E^* = E = 1,800,000 \text{ psi} = 1800 \text{ ksi}$$

$$E_{min-u} = E_{min} (K_F) = 660,000 \text{ psi} (1.765) = 1,165,000 \text{ psi} = 1165 \text{ ksi}$$

$$E_{min-u}^* = E_{min-u} (\Phi_s) = 1165 \text{ ksi} (0.85) = 990 \text{ ksi}$$

Design Of Wood Structures 6th Edition Solutions Manual

**Peter Niemz, Alfred Teischinger, Dick
Sandberg**



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topics like flitch beams wood poles used for footings reinforcement of existing structures and historical data on wood properties includes a section on long span and high rise wood structures includes an enhanced student design project

The Analysis of Irregular Shaped Structures Diaphragms and Shear Walls Terry R. Malone, Robert W. Rice, 2011-12-05 A Complete Guide to Solving Lateral Load Path Problems The Analysis of Irregular Shaped Structures Diaphragms and Shear Walls explains how to calculate the forces to be transferred across multiple discontinuities and reflect the design requirements on construction documents Step by step examples offer progressive coverage from basic to very advanced illustrations of load paths in complicated structures The book is based on the 2009 International Building Code ASCE SEI 7 05 the 2005 Edition of the National Design Specification for Wood Construction and the 2008 Edition of the Special Design Provisions for Wind and Seismic SDPWS 08 COVERAGE INCLUDES Code sections and analysis Diaphragm basics Diaphragms with end horizontal offsets Diaphragms with intermediate offsets Diaphragms with openings Open front and cantilever diaphragms Diaphragms with vertical offsets Complex diaphragms with combined openings and offsets Standard shear walls Shear walls with openings Discontinuous shear walls Horizontally offset shear walls The portal frame Rigid moment resisting frame walls the frame method of analysis **Engineering News-record**, 1903 **Contractor's**

Exam Book John Gladstone, George Perpich, Sandra Wackman Perpich, 1985 Building Physics of the Envelope Ulrich Knaack, Eddie Koenders, 2018-05-07 Die Fassade ist die Schnittstelle des Gebäudes mit seiner Umwelt Dort treffen bauphysikalische Parameter wie Wärme Feuchte Schall und Licht auf das Haus und wirken auf es ein Alle diese Einflüsse bedürfen der Regulierung durch die Gebäudehülle um Behaglichkeit für den Nutzer und Funktionstauglichkeit der Architektur zu gewährleisten Diese Einführung erläutert die wichtigsten Phänomene um dann den Bezug zur Baupraxis herzustellen Welche Materialien reagieren in welcher Weise auf diese Faktoren Wie gehen Fassadensystemen mit Wärme Feuchte Schall und Licht um Das praxisorientierte Buch entstanden aus der Zusammenarbeit eines Architekten und eines Bauingenieurs beschreibt die wichtigsten Fassadenmaterialien und Konstruktionen im Hinblick auf ihre bauphysikalische Performance

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The Athenaeum ,1864 **Athenaeum** James Silk Buckingham,John Sterling,Frederick Denison Maurice,Henry Stebbing,Charles Wentworth Dilke,Thomas Kibble Hervey,William Hepworth Dixon,Norman Maccoll,Vernon Horace Rendall,John Middleton Murry,1864 **Solid Wood** Joseph Mayo,2015-10-05 Over the past 10 15 years a renaissance in wood architecture has occurred with the development of new wood building systems and design strategies elevating wood from a predominantly single family residential idiom to a rival of concrete and steel construction for a variety of building types including high rises This new solid wood architecture offers unparalleled environmental as well as construction and aesthetic benefits and is of growing importance for professionals and academics involved in green design **Solid Wood** provides the first detailed book which allows readers to understand new mass timber massive wood architecture It provides historical context in wood architecture from around the world a strong environmental rationale for the use of wood in buildings recent developments in contemporary fire safety and structural issues insights into building code challenges detailed case studies of new large scale wood building systems on a country by country basis Case studies from the UK Norway Sweden Germany Austria Italy Canada the United States New Zealand and Australia highlight design strategies construction details and unique cultural attitudes in wood design The case studies include the most ambitious academic

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