Compression perpendicular to grain (bearing) $F_{-+-} = F_{-+}(K_F) = 565 \text{ psi } (2.083) = 1177 \text{ psi} = 1.177 \text{ ksi}$ $F_{c^{\perp}a^{-}} = F_{c^{\perp}a^{-}} (\Phi_{c})(\lambda) = 1.177 \text{ ksi } (0.9)(0.8) = 0.847 \text{ ksi}$ Compression parallel to grain $F_{cv} = F_c (K_F) = 1800 \text{ psi } (2.4) = 4320 \text{ psi} = 4.32 \text{ ksi}$ $F_{cn}' = F_{cn} (\Phi_c)(\lambda) = 4.32 \text{ ksi } (0.9)(0.8) = 3.11 \text{ ksi}$ $P_n' = F_{cn}' (A) = 3.11 \text{ ksi } (39.38 \text{ in}^2) = 122 \text{ k}$ Note that C_F would also apply for column buckling of an axially loaded member. Modulus of Elasticity $E^* = E = 1,800,000 \text{ psi} = 1800 \text{ ksi}$ $E_{\text{min},s} = E_{\text{min}}(K_F) = 660,000 \text{ psi} (1.765) = 1.165,000 \text{ psi} = 1165 \text{ ksi}$ $E_{\text{min-s}} = E_{\text{min-n}} (\Phi_s) = 1165 \text{ ksi } (0.85) = 990 \text{ ksi}$ 2 x 10 purlins at 4 ft o.c.; $1.2D + 1.6L_r$ load combination ($\lambda = 0.8$) C_F is already incorporated in NDS Table 4B values; other factors equal to unity $A = 13.88 \text{ in}^2$; $S_s = 21.39 \text{ in}^3$ Bending moment $F_{ha} = F_h (K_F) = 2050 \text{ psi } (2.54) = 5207 \text{ psi} = 5.21 \text{ ksi}$ $F_{ba}' = F_{ba} (\Phi_b)(\lambda) = 5.21 \text{ ksi } (0.85)(0.8) = 3.54 \text{ ksi}$ $M_n' = F_{bn}' (S_s) = 3.54 \text{ ksi } (21.39 \text{ in}^3) = 75.8 \text{ k-in}.$ Tension $F_{ts} = F_{r}(K_{F}) = 1100 \text{ psi } (2.7) = 2970 \text{ psi} = 2.97 \text{ ksi}$ $F_{to} = F_{to} (\Phi_t)(\lambda) = 2.97 \text{ ksi } (0.8)(0.8) = 1.90 \text{ ksi}$ $T_n' = F_m'(A) = 1.90 \text{ ksi } (13.88 \text{ in}^2) = 26.4 \text{ k}$ Flexural Shear $F_{vv} = F_v (K_F) = 175 \text{ psi } (2.88) = 504 \text{ psi} = 0.504 \text{ ksi}$ $F_{**}' = F_{**}(\Phi_*)(\lambda) = 0.504 \text{ ksi } (0.75)(0.8) = 0.302 \text{ ksi}$ $V_n' = F_{vn}'(2/3)(A) = 0.302 \text{ ksi } (2/3)(13.88 \text{ in}^2) = 2.8 \text{ k}$ Compression perpendicular to grain (bearing) $F_{c^{\perp n}} = F_{c^{\perp}}(K_F) = 565 \text{ psi } (2.083) = 1177 \text{ psi} = 1.177 \text{ ksi}$ $F_{-4\alpha}$ = $F_{-4\alpha}$ (Φ_{c})(λ) = 1.177 ksi (0.9)(0.8) = 0.847 ksi Compression parallel to grain $F_{cv} = F_c (K_F) = 1850 \text{ psi } (2.4) = 4440 \text{ psi} = 4.44 \text{ ksi}$ $F_{cn}' = F_{cn}(\Phi_c)(\lambda) = 4.44 \text{ ksi } (0.9)(0.8) = 3.20 \text{ ksi}$ $P_n' = F_{cn}' (A) = 3.20 \text{ ksi } (13.88 \text{ in}^2) = 44.4 \text{ k}$ Note that Cr would also apply for column buckling of an axially loaded member. Modulus of Elasticity $E^* = E = 1,800,000 \text{ psi} = 1800 \text{ ksi}$ $E_{min,\sigma} = E_{min}(K_F) = 660,000 \text{ psi}(1.765) = 1,165,000 \text{ psi} = 1165 \text{ ksi}$ $E_{\text{min,o}} = E_{\text{min,o}} (\Phi_i) = 1165 \text{ ksi } (0.85) = 990 \text{ ksi}$

C.

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Rose Arny

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topics like flitch beams wood poles used for footings reinforcement of existing structures and historical data on wood properties includes a section on long span and high rise wood structures includes an enhanced student design project

The Analysis of Irregular Shaped Structures Diaphragms and Shear Walls Terry R. Malone, Robert W. Rice, 2011-12-05 A Complete Guide to Solving Lateral Load Path Problems The Analysis of Irregular Shaped Structures Diaphragms and Shear Walls explains how to calculate the forces to be transferred across multiple discontinuities and reflect the design requirements on construction documents Step by step examples offer progressive coverage from basic to very advanced illustrations of load paths in complicated structures The book is based on the 2009 International Building Code ASCE SEI 7 05 the 2005 Edition of the National Design Specification for Wood Construction and the 2008 Edition of the Special Design Provisions for Wind and Seismic SDPWS 08 COVERAGE INCLUDES Code sections and analysis Diaphragm basics Diaphragms with end horizontal offsets Diaphragms with intermediate offsets Diaphragms with openings Open front and cantilever diaphragms Diaphragms with vertical offsets Complex diaphragms with combined openings and offsets Standard shear walls Shear walls with openings Discontinous shear walls Horizontally offset shear walls The portal frame Rigid moment resisting frame walls the frame method of analysis **Engineering News-record**, 1903 **Exam Book** John Gladstone, George Perpich, Sandra Wackman Perpich, 1985 Building Physics of the Envelope Ulrich Knaack, Eddie Koenders, 2018-05-07 Die Fassade ist die Schnittstelle des Geb udes mit seiner Umwelt Dort treffen bauphysikalische Parameter wie W rme Feuchte Schall und Licht auf das Haus und wirken auf es ein Alle diese Einfl sse bed rfen der Regulierung durch die Geb udeh lle um Behaglichkeit fr den Nutzer und Funktionst chtigkeit der Architektur zu gew hrleisten Diese Einf hrung erl utert die wichtigsten Ph nomene um dann den Bezug zur Baupraxis herzustellen Welche Materialien reagieren in welcher Weise auf diese Faktoren Wie gehen Fassadensystemen mit W rme Feuchte Schall und Licht um Das praxisorientierte Buch entstanden aus der Zusammenarbeit eines Architekten und eines Bauingenieurs beschreibt die wichtigsten Fassadenmaterialien und konstruktionen im Hinblick auf ihre bauphysikalische Performance

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